

The Open University of Sri Lanka
Faculty of Engineering Technology
Department of Civil Engineering



Study Programme	: Bachelor of Technology Honours in Engineering
Name of the Examination	: Final Examination
Course Code and Title	: CVX4348 - Water and Wastewater Engineering
Academic Year	: 2024/2025
Date	: 6 th March 2026
Time	: 0930 hrs-1230hrs
Duration	: 3 hours

General Instructions

1. Read all instructions carefully before answering the questions.
 2. This question paper consists of **seven (7)** questions in **six (6)** pages.
 3. Answer any **Five (5)** questions only. All questions carry Twenty (20) equal marks.
 4. Answer for each question should commence from a new page.
 5. Relevant charts / codes / equations are provided in last page
 6. This is a Closed Book Test **(CBT)**.
 7. Answers should be in clear handwriting.
 8. Do not use the red colour pen.
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Question 1**[20 Marks]**

- (a) Population forecasting is required to estimate future water demand for planning and design.
- (i) Define design period and design population in water supply engineering.
- (ii) State two reasons why water treatment plants are usually designed for long design periods, but distribution networks may be upgraded in stages.
- (iii) A township plans a new water treatment plant with a 30-year design period. Population data is:

Year	1990	2000	2010	2020
Population	3200	5100	7400	10600

Estimate the population in 2050 using a suitable method, and determine the average daily design flow if per capita demand is 220 Lpcd.

- (b) An intake structure must ensure reliable withdrawal of water under varying source conditions.
- (i) List five operational and water-quality factors that influence intake design (not only location).
- (ii) With sketches, differentiate river intake and reservoir intake and explain one advantage and one limitation of either type.
- (c) (i) List four pressure-driven membrane processes used in water treatment.
- (ii) Explain the working principle of nanofiltration (NF) OR reverse osmosis (RO).
- (iii) Give four causes of taste and odour in drinking water (biological/chemical/physical).
- (iv) List three practical applications which are used to control taste and odour in water treatment plants.

Question 2**[20 Marks]**

- (a) (i) List four important water-quality parameters tested in the lab that are directly relevant to sedimentation and filtration design.
- (iii) A WTP must treat 22 ML/d. Design a circular sedimentation tank for a surface overflow rate of $28 \text{ m}^3/\text{m}^2\cdot\text{d}$.
- If detention time is 2.0 h and side water depth is 3.5 m, determine:
- required surface area
 - tank diameter
 - tank volume

- (b) (i) State two objectives of filtration in drinking-water treatment.
 (ii) A rapid gravity filter has plan dimensions $7\text{ m} \times 5\text{ m}$. It receives 4.2 million L/d. Find the filtration rate (m/h).
 (iii) If the backwash rate is $45\text{ m}^3/\text{h}/\text{m}^2$ and backwash duration is 7 minutes, determine the backwash water volume per wash.
 (iv) Estimate the percentage of daily backwash water quantity out of total treated water per day.
- (c) (i) A plant targets 0.5 mg/L free chlorine residual after contact. If chlorine demand is 1.2 mg/L , estimate required applied dose.
 (ii) Why is backwashing necessary in rapid sand filters?
 (iii) A filter unit has an area of 60 m^2 . If it is operated at 6 m/h , find the treated water output in ML/d.
 (iii) A plant applies 80 kg/day of chlorine to disinfect 16 ML/d . Compute the dosage in mg/L .

Question 3**[20 Marks]**

- (a) (i) Why is a distribution reservoir necessary for a water distribution system. Briefly explain.
 (ii) What are the advantages of gate valves which use for water distribution networks. List three of them.
 (iii) To distribute water among consumers a distribution system net work is used. Name four different distribution systems which are designed based on the location of buildings.
- (b) (i) List two advantages of using inclined plate (lamella) settlers.
 (iii) A rectangular clarifier is designed for flow 30 ML/d and overflow rate $32\text{ m}^3/\text{m}^2\cdot\text{d}$. Assume $L:W = 4:1$ and depth = 3.0 m . Determine the required length and width, and the volume.
- (c) (i) A city water treatment plant needed to treat 24 ML/d . Assuming the losses and wastages are about 15% estimate the amount of alum needed in Kg/day for the treatment plant if the dosage is 14 mg/L .
 (ii) Activated carbon is commonly used as an advanced treatment step. State two locations in a Water Treatment Plant where PAC dosing may be applied and why.

Question 4.**[20 Marks]**

- (a) (i) What is the consequence of underestimating T_c in storm sewer design?
- (ii) A road drainage catchment has $A = 0.8 \text{ km}^2$, $C = 0.80$, and rainfall intensity $I = 90 \text{ mm/h}$. Compute peak runoff using Rational Method.
- (iii) A pipe of 600 mm diameter is laid at a slope that gives full-flow velocity 1.6 m/s . Check whether this pipe can carry the computed flow at full flow.

- (b) A catchment has the following land-use distribution:

Type	% Area	Runoff coefficient (C)
Roofs	18	0.90
Roads/Pavements	22	0.85
Lawns/Parks	35	0.20
Compacted soil	15	0.35
Wooded area	10	0.10

- (i) Determine the weighted average runoff coefficient for the catchment.
- (ii) If the catchment area is 30 ha and rainfall intensity is 65 mm/h, compute peak runoff using Rational formula $Q = 0.278 C I A$ (Q in m^3/s , I in mm/h , A in km^2).
- (c) (i) What happens to DO immediately after a discharge point, and why can DO recover downstream?
- (ii) Draw a labelled oxygen sag curve using time from discharge point on the x-axis. Show the critical point and explain its significance.
- (iii) List four mechanisms contributing to river self-purification (physical/chemical/biological).

Question 5**[20 Marks]**

- (a) (i) Explain crown corrosion in sewers (mechanism) with a neat sketch.
- (ii) List four methods to prevent or reduce crown corrosion.
- (iii) Why is ventilation important in large sewer systems?
- (b) (i) Compare Activated Sludge and Trickling Filters in terms of: land requirement, energy use, process stability (any three).
- (ii) The BOD_5 of waste has been measured as 200 mg/L . If the reaction rate constant is $k = 0.21/\text{day}$, what would be the ultimate BOD of waste?

- (c) Two circular secondary clarifiers are proposed, each of diameter 12 m and water depth 3.0 m, treating 2400 m³/day. Calculate,
- Detention time,
 - Surface overflow rate,
 - Weir loading if each tank has a peripheral weir.

Question 6**[20 Marks]**

- (a) (i) List four forces/loads acting on an underground sewer pipe.
 (ii) Explain any one of the above loads in detail and how it influences design.
 (iii) Define self-cleansing velocity and state two typical problems caused when it is not achieved.
- (b) A food processing factory discharges 1500 m³/day. After equalization and primary settling, the BOD₅ entering aeration is ($S_0=780$) mg/L. Effluent BOD₅ is ($S=35$) mg/L. Aeration tank = 10 m × 8 m × 4.5 m. MLSS ($X = 3600$) mg/L. Sludge age $\theta_c = 18$ days. Determine:
- (i) Aeration period HRT in hours
 - (ii) F/M ratio
 - (iii) BOD removal efficiency across biological treatment
 - (iv) Oxygen requirement per day
- (c) (i) An aeration tank treats 6000 m³/day of wastewater with influent BOD₅ = 220 mg/L. Tank volume = 1800 m³, MLSS = 3000 mg/L. Calculate F/M ratio.
 (ii) Briefly discuss how changing F/M, affects sludge settleability and effluent quality.

Question 7**[20 Marks]**

- (a) (i) What is meant by sludge bulking? State two causes.
 (ii) Explain the significance of:
- F/M ratio
 - SRT (sludge age)
 - DO concentration
 - MLSS/MLVSS
- in controlling performance and preventing sludge bulking.

- (b) (i) Differentiate between aerobic and anaerobic digestion.
 (ii) List three benefits of equalization for industrial wastewater (flow + quality).
 (iii) Why is pH control important before biological treatment? Mention two impacts of improper pH.

(c) Primary clarifier design: overflow rate + weir loading

A treatment plant handles 15,000 m³/day. Two rectangular primary clarifiers are planned in parallel.

Design criteria:

- Surface overflow rate = 35 m³/m²·day
- Detention time = 2.5 h
- Depth = 3.5 m
- Each tank provides two end weirs, each equal to tank width
- Assume L:W = 5:1

Determine:

- (i) Total surface area required and area per tank
 (ii) Dimensions (L and W) of each tank
 (iii) Detention time
 (iv) Weir loading rate

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Supplementary

$$k = k_{20} \vartheta^{(T-20)}; \quad BOD_5 = L_0 (1 - e^{-kt}); \quad \vartheta = 1.047$$

$$r_u = -\frac{\mu_m S X_{avg}}{K_s + S}$$

$$\frac{1}{\theta_c} = \frac{Y \mu_m (S_0 - S)}{(S_0 - S) + (1 + \alpha) K_s \ln(S_i/S)} - k_d$$

$$\frac{F}{M} = \frac{Q S_0}{V X}$$

$$\text{Oxygen requirement} = 1.47 Q (S_0 - S) - 1.42 V (x/\theta_c)$$