



CEX6230 – GEOTECHNICS

Time allowed: Three Hours

Date: Thursday, 6<sup>th</sup> April, 2006

Time: 0930-1230

You may attach the last sheet to your script.

Answer five questions. All questions carry equal marks.

1. Rain induced slope failures are common in the central hill country. It is observed that unstable soil masses move faster during the rainy season compared to little or no movement during the dry season.

- Explain what makes the soil mass to loose its shear strength when potential slip surface has saturated conditions. (5 points)
- Explain how subsurface drainage could improve stability. (3 points)
- During a field investigation you may see signs that may lead to a future slope failure.
  - List three site conditions leading to such failures. (3 points)
  - List three signs of distress that may show imminent danger. (3 points)
- Figure Q1 shows a proposed highway cut. Using Taylor's Stability Chart, determine the factor of safety,  $F$ .  
 $N_s$  is defined as  $N_s = \frac{c_u}{F\gamma H}$ . (6 points)

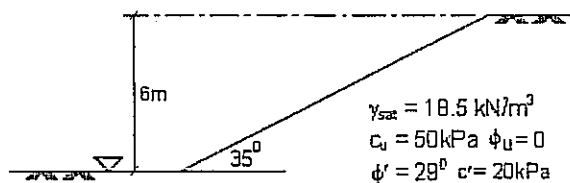


Figure Q1

2. Figure Q2 shows a cross-section of a long clay dam drawn to scale.

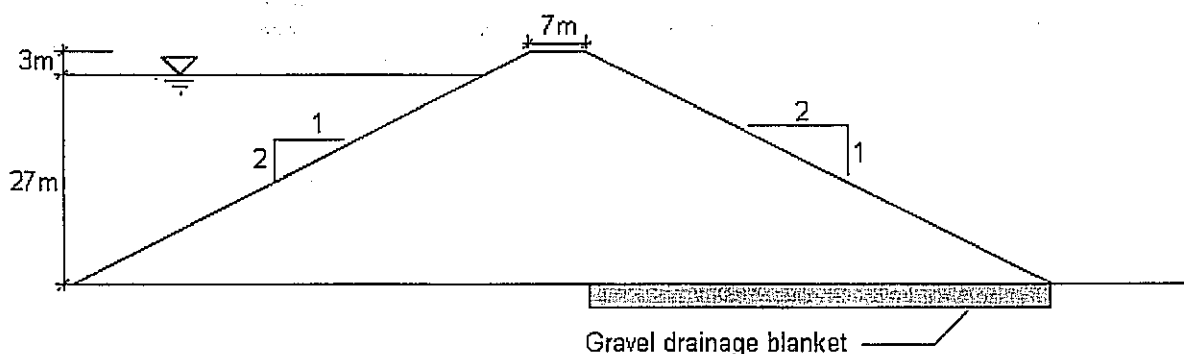


Figure Q2

- Determine the head (in meters) that causes flow through the dam. (2 points)
- State which head (i.e. the heads: pressure, velocity, elevations or the total head) that forces water to flow through sand. (1 point)
- Draw the phreatic surfaces. (4 points)
- Draw the flow net paying attention to specific rules that you have learnt in sketching a flow net. (8 points)

- e) Compute the seepage rate (in  $\text{m}^3/\text{s}$  per meter run) considering that the coefficient of permeability of clay is  $0.0000015 \text{ cm/s}$ . (5 points)
3. Figure Q3 shows a soil profile where the sand layer at point C is under artesian pressure.
- Draw the total stress, pore water pressure and effective stress distribution with depth, indicating principal values. (10 points)
  - Suppose that the artesian pressure drops to  $0.5\text{m}$  below ground surface due to pumping, determine the effective stress distribution in the clay layers. (6 points)
  - Determine the resulting effect of the pressure drop on the clay layer. (4 points)

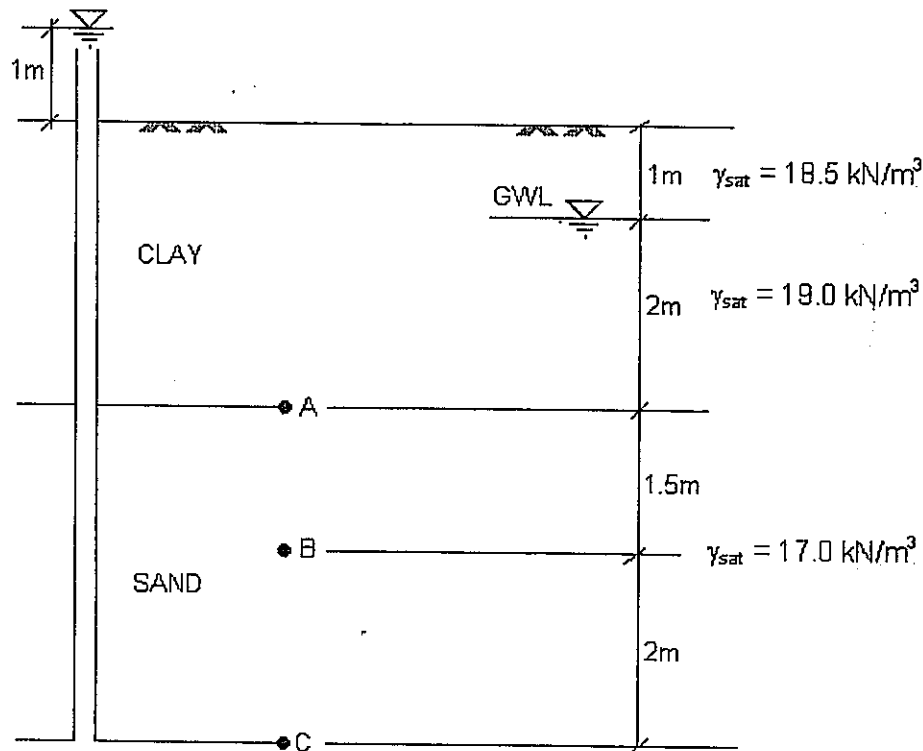


Figure Q4

4. In a triaxial loading test, the soil specimen was allowed to consolidate completely under a cell pressure of  $200\text{kPa}$ . The cell pressure was then increased to  $350\text{kPa}$  while keeping the drains closed. The corresponding increase in pore water pressure was found to be  $144\text{kPa}$ . The sample was strained without allowing drainage. The results are shown in the table below.

a) Complete the table below:

Axial Strain (%)	Deviatoric Stress (kPa)	$u$ (kPa)	$\sigma_1$ (kPa)	$\sigma'_1$ (kPa)	$q$ (kPa)	$p$ (kPa)	$p'$ (kPa)
0	0	144					
2	201	244					
4	252	240					
6	275	222					
8	282	212					
10	283	209					

- Plot the variation of  $p - q$  and  $p - q'$  on the same plot. (6 points)
- Calculate the parameter  $B$ . (4 points)
- Assuming that the soil is normally consolidated, compute the shear strength parameters based on effective stresses; estimate the shear strength parameters based on total stresses. (2 points)
- Assuming that the soil is normally consolidated, compute the shear strength parameters based on effective stresses; estimate the shear strength parameters based on total stresses. (5 points)

- e) State whether the plot supports your assumption; give your reasons. (3 points)

5. Candidates are required make their answers brief and to the point.

- a) Sketch a typical plot obtained during a pile load test; list the type of information that are used to verify pile load capacity. (4 points)
- b) Differentiate between individual failure and block failure in a pile group. (4 points)
- c) Explain the concept of negative skin friction. (4 points)
- d) End bearing and shaft resistance of a pile is expressed as  $9c_u A_b$  and  $[c_a + K_s \gamma D \tan \delta] A_s$ , respectively. Define the terms used in these equations. (4 points)
- e) Drained stress-strain behaviour of sands shows that during shearing, loose sand reduces its volume while dense sand dilates. It is also observed that dense sand passes through its peak strength before attaining a residual value. Explain this behaviour. (4 points)

6. Candidates are required make their answers brief and to the point.

- a) Table below shows the results of a series of Unconsolidated Undrained loading tests.

Cell pressure (kPa)	200	400	600
Deviatoric stress at failure (kPa)	222	218	220

- i) Determine shear strength parameters  $c_u$  and  $\phi_u$ . (4 points)
- ii) State what additional information is required for you to determine  $\phi'$ . (2 points)
- b) The coefficients of earth pressure  $K_0$ ,  $K_a$  and  $K_p$  defines the relationship between lateral and vertical effective stress at a point.
- i) Explain the  $K_0$  condition. (3 points)
- ii) For a soil with  $\phi = 30^\circ$ , determine the three K values. (5 points)
- c) Bishop's Pore Pressure equation is expressed as  $\Delta u = B[\Delta \sigma_3 + A(\Delta \sigma_1 - \Delta \sigma_3)]$ .
- i) Explain the significance of parameter B. (3 points)
- ii) Explain the significance of parameter A. (3 points)

7. Figure Q7(a) shows the variation of pressure on a wall with wall movement.

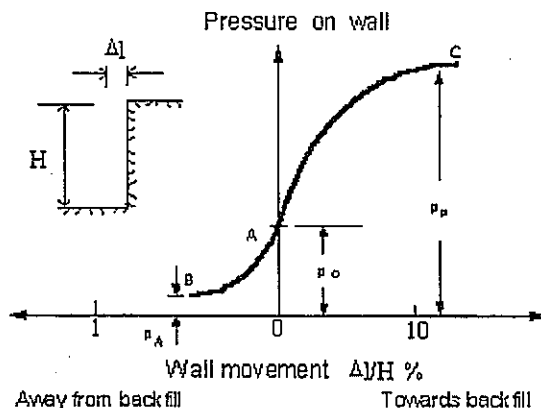


Figure Q7(a)

- a) State the conclusion you have drawn from the information given in Figure Q7(a) (5 points)
- b) Figure Q7(b) shows a braced excavation made on a coarse grained soil.
- i) Determine the force per unit length acting on the three struts. To make the analysis simpler you may consider that two sheet piles are connected at the central strut, by a hinge. (12 points)
- ii) Explain how you intend to control seepage and piping prior to concreting. (3 points)

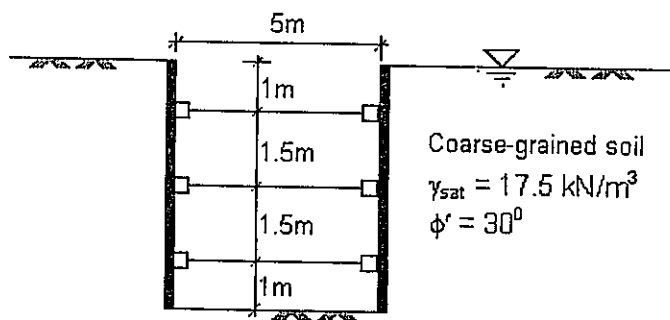


Figure Q7(b)

8. Pad foundation shown in Figure Q8 is to be checked for stability.
- Determine foundation width for the square footing for a factor of safety of 2.5 to meet 25mm settlement requirement. (10 points)
  - Determine the consolidation settlement. (10 points)

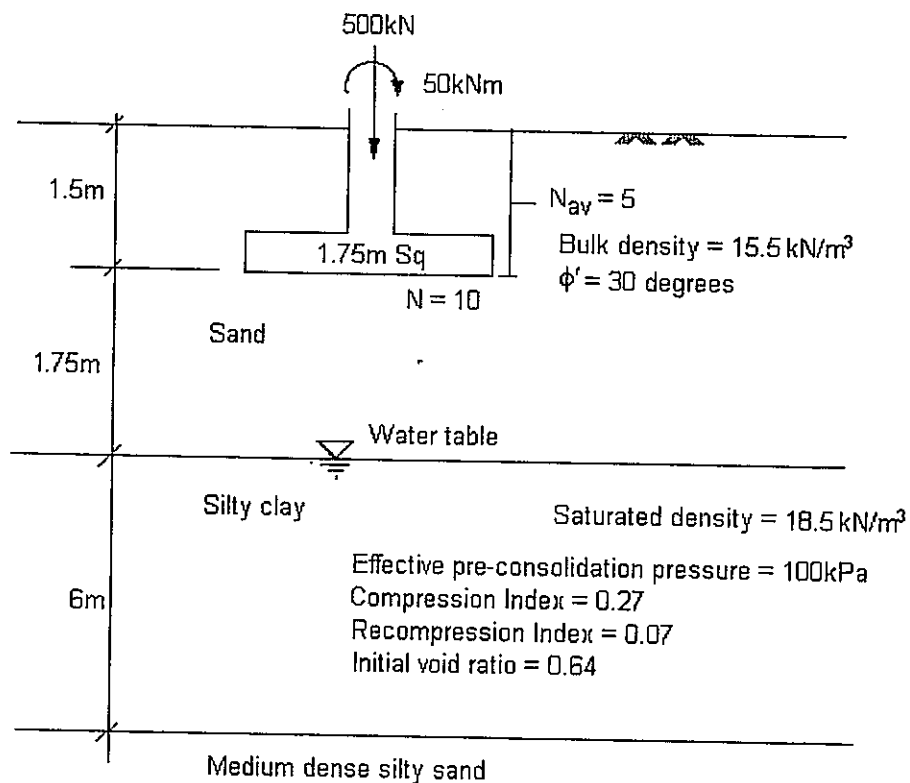
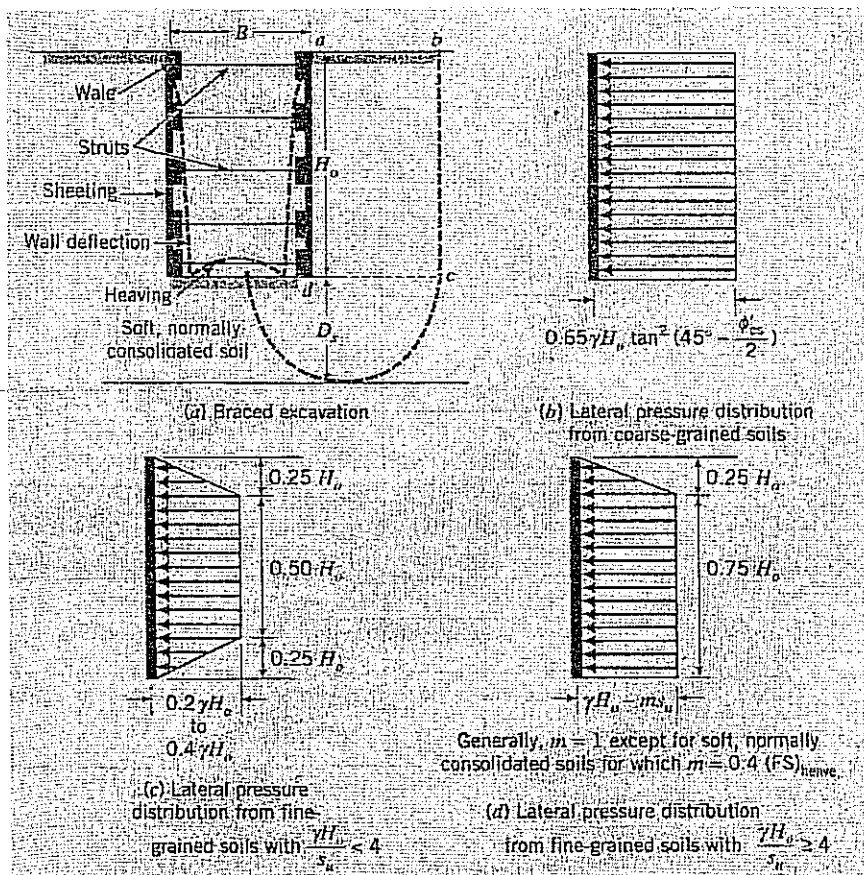
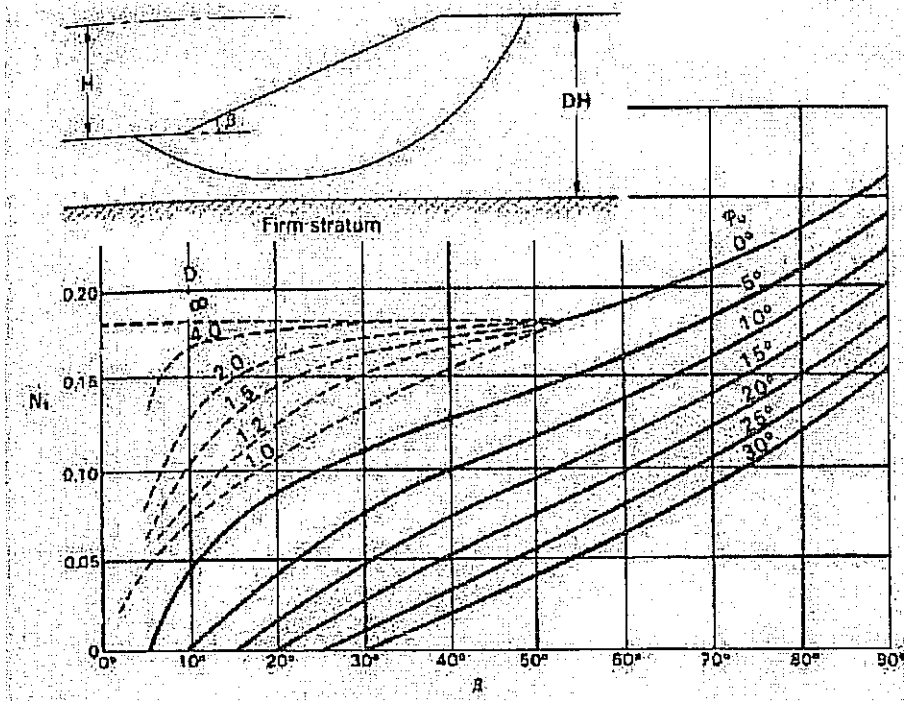
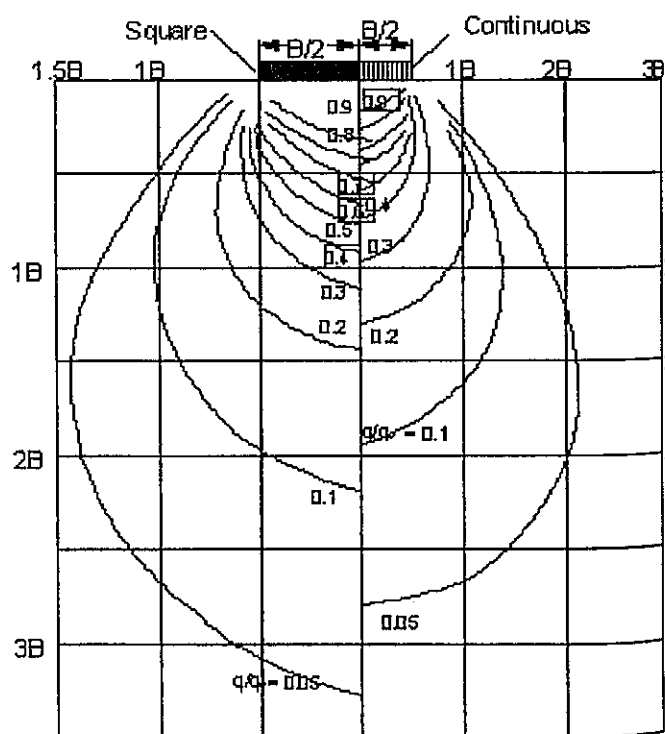
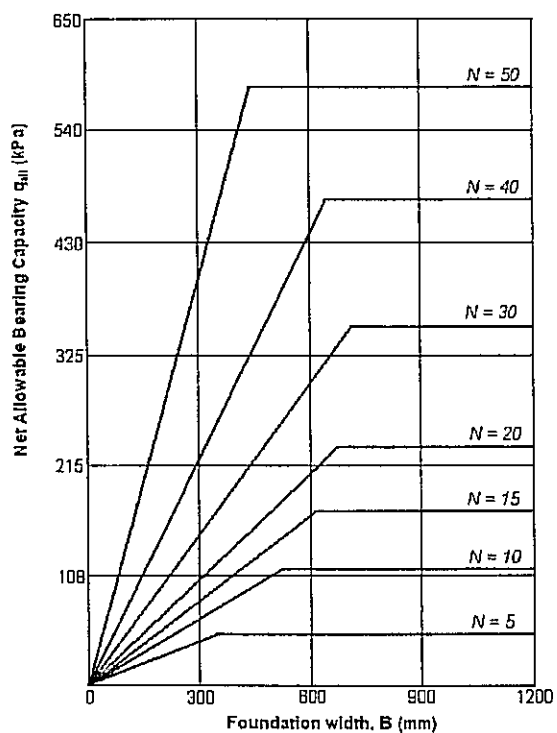
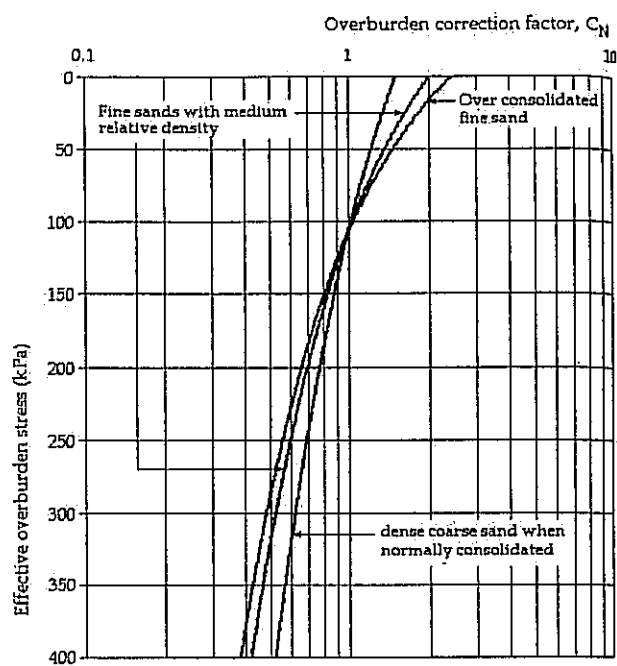
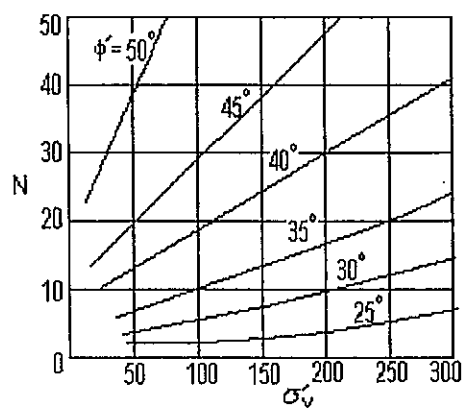
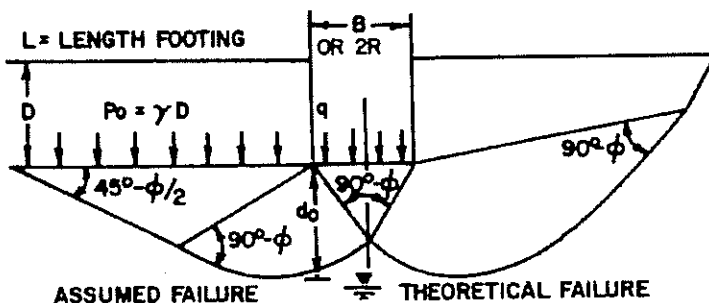
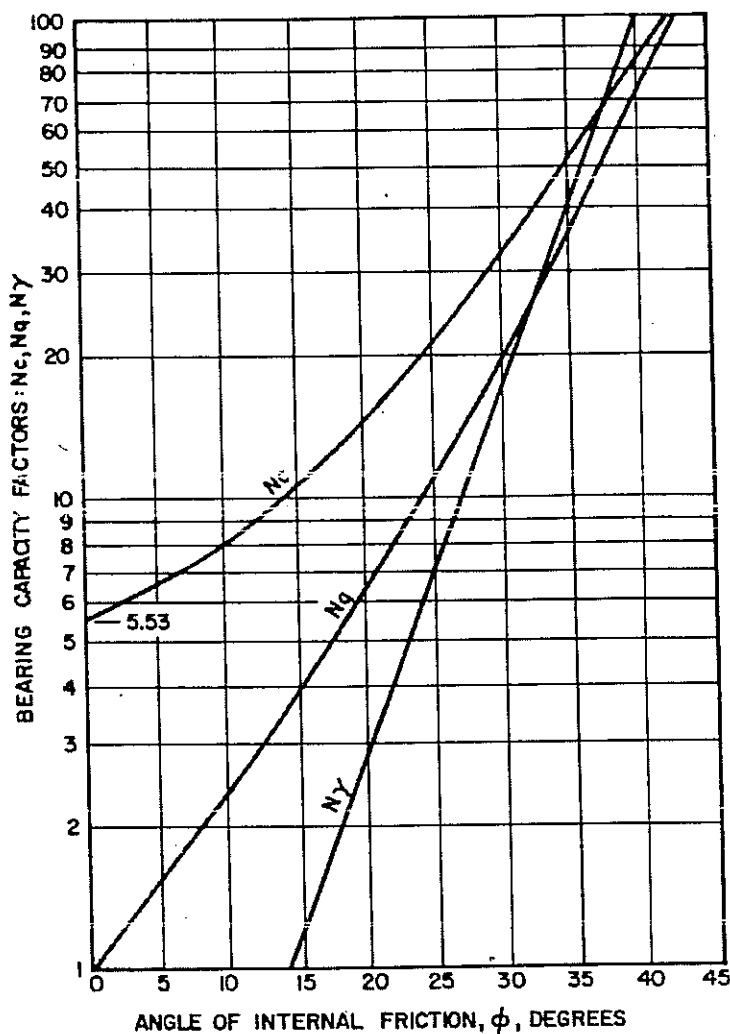


Figure Q8

tlement  
(10 point  
(10 point







ASSUMED CONDITIONS:

1.  $D \leq B$
2. SOIL IS UNIFORM TO DEPTH  $d_0 > B$ .
3. WATER LEVEL LOWER THAN  $d_0$  BELOW BASE OF FOOTING.
4. VERTICAL LOAD CONCENTRIC.
5. FRICTION AND ADHESION ON VERTICAL SIDES OF FOOTING ARE NEGLECTED.
6. FOUNDATION SOIL WITH PROPERTIES  $c, \phi, \gamma$

ULTIMATE BEARING CAPACITY =  $q_{ult}$

CONTINUOUS FOOTING; GENERAL CASE

$$q_{ult} = q' + q''$$

$q' =$  PORTION OF BEARING

CAPACITY ASSUMING

WEIGHTLESS FOUNDATION SOIL

$q'' =$  PORTION OF BEARING

CAPACITY FROM WEIGHT OF

FOUNDATION SOILS

$$q' = cN_c + \gamma D N_q$$

$$q'' = \gamma \frac{B}{2} N_\gamma$$

$$q_{ult} = cN_c + \gamma D N_q + \frac{\gamma B}{2} N_\gamma$$

SQUARE OR RECTANGULAR FOOTING

$$q_{ult} = cN_c \left(1 + 3 \frac{B}{L}\right) + \gamma D N_q + 0.4 \gamma B N_\gamma$$

CIRCULAR FOOTING:  $R = B/2$

$$q_{ult} = 1.3 cN_c + \gamma D N_q + 0.6 \gamma R N_\gamma$$

FOR COHESIONLESS FOUNDATION SOILS ( $c = 0$ )

CONTINUOUS FOOTING:

$$q_{ult} = \gamma D N_q + \frac{\gamma B}{2} N_\gamma$$

SQUARE OR RECTANGULAR FOOTING:

$$q_{ult} = \gamma D N_q + 0.4 \gamma B N_\gamma$$

CIRCULAR FOOTING:

$$q_{ult} = \gamma D N_q + 0.6 \gamma R N_\gamma$$

FOR COHESIVE FOUNDATION SOILS ( $\phi = 0$ )

CONTINUOUS FOOTING:

$$q_{ult} = cN_c + \gamma D$$

SQUARE OR RECTANGULAR FOOTING:

$$q_{ult} = cN_c \left(1 + 3 \frac{B}{L}\right) + \gamma D$$

CIRCULAR FOOTING:

$$q_{ult} = 1.3 cN_c + \gamma D$$

FIGURE 1

Ultimate Bearing Capacity of Shallow Footings With Concentric Loads

✂ Attach this sheet to your answer script

Index No. \_\_\_\_\_

