



Time allowed: Three Hours.

Index No. _____

Date: Friday, 16th March, 2012

Time: 1400-1700

PART A:

Answer all questions. All questions carry equal marks. Attach Part A of this paper to your answer script. You are advised to spend approximately One (1) hour for Part A. State your response within the space provided. (3x12 = 36 points)

1. Which of the following statements are true?

- A. 100g of mineral Kaolinite adsorbs less water than 100g of mineral Montmorillonite.
- B. Plastic limit is the consistency limit between semi-liquid and semi-solid states.
- C. A 'ML' soil is a silty soil.
- D. A 'CH' soil should have Plasticity Index greater than 20.

- a) A, B and C only b) A, B and D only c) B, C and D only d) A, C and D only
- e) A, B, C and D.

2. A saturated clay soil specimen has the following properties:

Natural water content	= 60%
Void Ratio	= 0.67
Liquid Limit	= 82
Plastic Limit	= 35

Its Liquidity Index is:

- a) 0.47 b) 0.53 c) 0.68 d) 0.73 e) 0.77

3. Bulk Density of a soil is expressed as $\rho = \frac{G_s + Se}{X} \cdot \rho_w$. The denominator, X is equal to:

- a) $1 + w$ b) $1 + n$ c) $1 + S$ d) $1 + e$ e) n

4. A sandy soil is subjected to a consolidated drained triaxial loading test, at a constant cell pressure of 100kPa. The deviatoric stress at failure is found to be 225kPa. The friction angle ϕ' is equal to:

- a) 32 b) 33 c) 34 d) 35 e) 36

5. Which of the following statements are true regarding Terzaghi's 1-dimensional consolidation theory?

- A. During initial compression it is considered that excess pore water pressure dissipation takes place.
- B. Time taken for primary consolidation to occur depends on the permeability of the soil.
- C. Degree of consolidation measures primary consolidation settlement at a given time after placing the load.
- D. A plot of settlement versus logarithmic time is used to estimate settlement at zero excess pore water pressure.

- a) A, B and C only b) A, B and D only c) B, C and D only d) A, C and D only
- e) A, B, C and D.

6. The results obtained during a Standard Proctor Compaction test specimen are given below:

Weight of soil + mould (g)	= 3921.4
Weight of mould (g)	= 1933.0
Average water content (%)	= 10.1
Volume of mould (c.c.)	= 1000
G_s	= 2.65

The dry density in kN/m^3 is equal to:

- a) 17.7 b) 18.1 c) 18.7 d) 19.2 e) 19.6

7. Which of the following statements are true?

- A. Coefficient of permeability is measured in cm^2/sec .
- B. Coefficient of permeability increases with fluid viscosity.
- C. 'Sand boiling' occurs at a hydraulic gradient of around 1.0.
- D. Darcy's law is applicable only for laminar flow situations.

- a) A and B only b) A and C only c) A and D only d) B and C only
e) C and D only

8. Angle of repose of a cohesionless granular soil does not depend on:

- a) Surface roughness of soil grains.
- b) Size distribution of particles in the soil heap.
- c) Shape of soil grains.
- d) Moisture content.
- e) Complete submergence of soil heap.

9. A material with $\gamma' = 12 \text{ kN/m}^3$, $\phi' = 0$ and $c' = 0$ is retained by a vertical wall of 5m height. The force exerted per 1m of wall length is:

- a) 50kN b) 100kN c) 150kN d) 200kN e) 450kN

10. Which of the following statements concern the Unconfined Compression Test?

- A. The test yields the shear strength parameter c' .
- B. The test is performed on cohesive soils only.
- C. The test gives a zero friction angle.
- D. The test is performed on fully saturated specimens only.

- a) A, B and C only b) A, B and D only c) B, C and D only d) A, C and D only
e) A, B, C and D.

11. A well graded sandy soil specimen has 10% fines. Which of the following statements are true?

- A. Unified Soil Classification System (USCS) classifies the soil as SW.
- B. USCS requires us to determine the type of fines.
- C. USCS recommends Liquid and Plastic Limit tests to determine the type of fines.
- D. The soil may be classified as SC.

- a) A and B only b) A and C only c) A and D only d) B and C only
e) B and D only

12. Which of the following statements are true?

- A. Auger boring with periodic withdrawal of material is suitable for soft clays.
- B. Rotary drilling removes cutting using circulating drilling fluid.
- C. Standard Penetration Test uses a split barrel sampler.
- D. Undisturbed samples are obtained by driving a tube sampler.

- a) A, B and C only b) A, B and D only c) B, C and D only d) A, C and D only
e) A, B, C and D.

PART B:

Answer four questions. All questions carry equal marks. You are advised to spend approximately 28 minutes per question. (16x4 = 64 points)

1. Figure B1 shows compaction curves observed for various soil types.

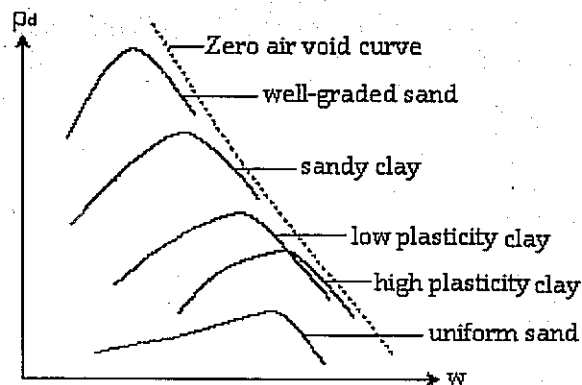


Figure B1

- Derive the relationship for dry density, ρ_d in terms of degree of saturation, S , specific gravity, G_s and water content, w . (4 points)
- Sketch to the same approximate scale, the 3-phase diagrams for:
 - A soil element, which is wet of optimum moisture content. (2 points)
 - A soil element, which is completely saturated. (2 points)
- Discuss the relative changes in each phase for the two cases discussed in question 1(b) above. (2 points)
- Explain why dry density decreases with moisture content, at wet of optimum water content. (2 points)
- Explain why well-graded sands have higher dry densities than uniformly graded sands. (2 points)
- Explain why clays with low and high plasticity have their respective compaction curves closer to the zero air void curve. (2 points)

2. Figure B2 shows a retaining wall of a waterfront structure.

- Draw the variation of i) total vertical stress ii) hydro-static stress and iii) vertical effective stress, with depth. State principal values. (4 points)

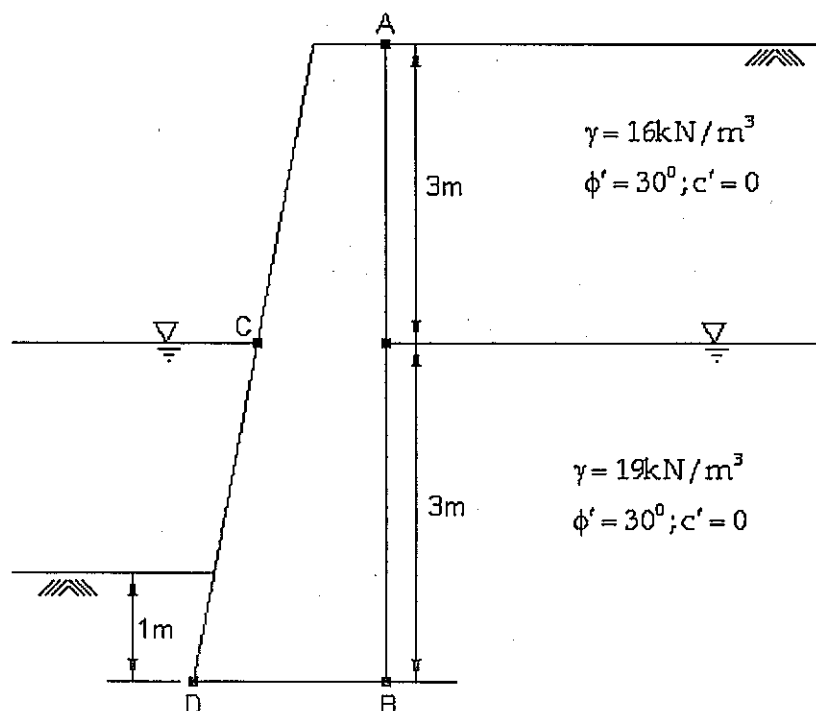


Figure B2

- b) Draw the effective horizontal stress distribution acting on smooth wall, along a meter length of AB, if 'at-rest' condition (i.e. K_0 condition) exists. State principal values. Hence compute the total force per meter length of wall, exerted by the retained soil. (6 points)
- c) State the forces that prevent sliding along base DB. (3 points)
- d) State the forces that prevent overturning about point D. (3 points)
3. Figure B3 shows a sloping ground AB, retaining a soil height of 3m. AC represents a trial failure plane along which soil wedge ABC may slide. Soil properties are $\gamma = 16 \text{ kN/m}^3$, $\phi' = 28^\circ$, and $c' = 20 \text{ kN/m}^2$. The respective factors of safety are expressed as: $F_\phi = \frac{\tan \phi'}{\tan \phi'_d}$; $F_c = \frac{c'}{c'_d}$.

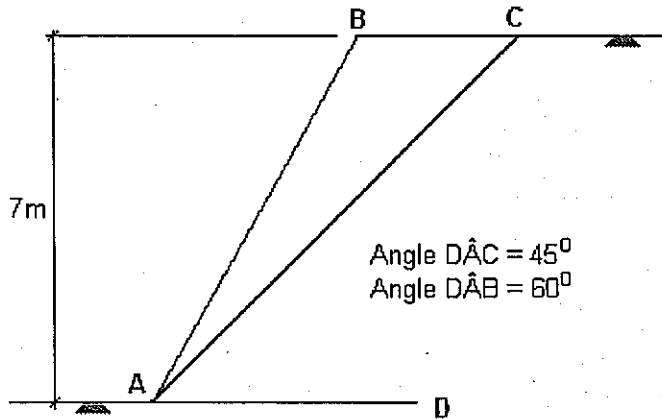


Figure B3

- a) Define the terms stated in the two equations representing factors of safety. (4 points)
- b) Compute the normal stress, σ_n acting on plane AC. (4 points)
- c) Compute the shear stress, τ acting on plane AC (4 points)
- d) Hence compute the factor of safety against shear failure. (4 points)
4. Figure B4(a) shows a plot of $e - \log \sigma'_v$ obtained during an Oedometer test. The sample had an initial height, $H_0 = 20 \text{ mm}$; average diameter = 50mm; $e_0 = 2.93$; total settlement, $\Delta H = 2.705 \text{ mm}$.

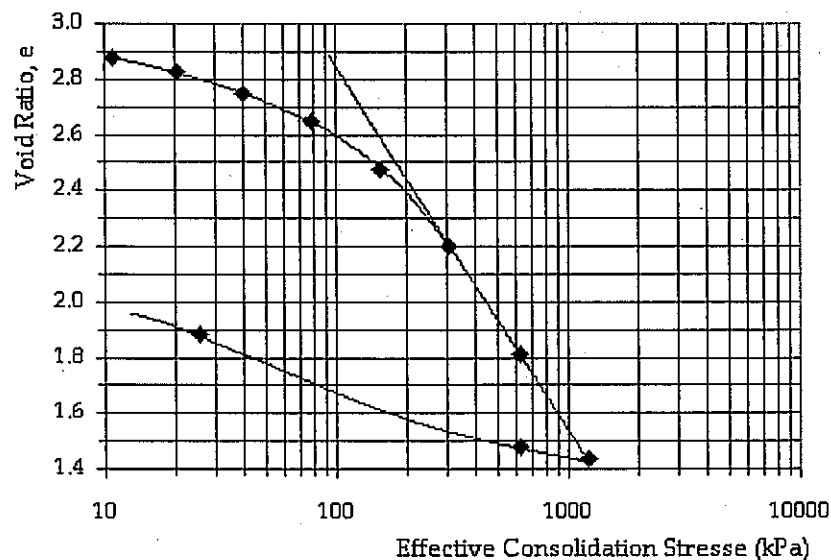


Figure B4(a)

- a) Determine the Compression Index and Recompression Index. (5 marks)
- b) Determine the Pre-consolidation pressure. (3 marks)
- c) Figure B4(b) shows the 16m thick clay layer subject to a 1.5m surcharge fill. The above mentioned specimen is obtained from a 8m depth below ground level.
- i) Compute the effective vertical overburden stress. (3 marks)
- ii) State whether the soil is Normally Consolidated or Over Consolidated. (2 marks)
- d) Estimate the expected total settlement of the clay layer. (3 marks)

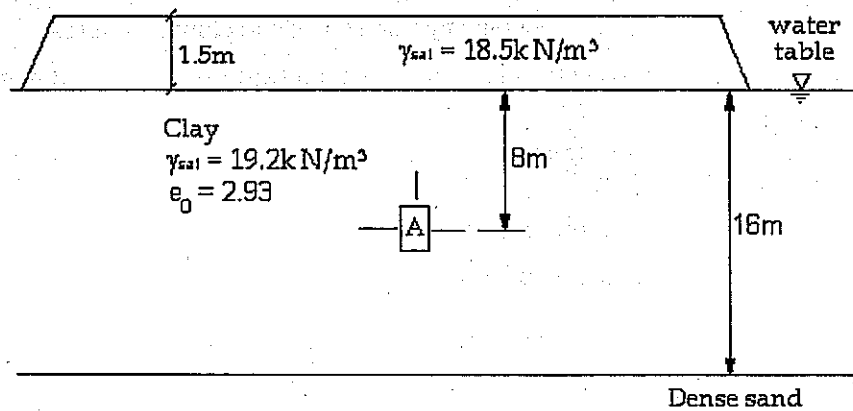


Figure B4(b)

5.

- a) Suppose that you are given a tube sample containing a fine-grained soil; list the standard tests you would perform in order to classify the soil based on Unified Soil Classification System (USCS). (3 points)
- b) Explain how you would determine its plasticity characteristics based on USCS. (3 points)
- c) Table below lists results of a laboratory test specimen:

Particle size (mm)	% passing
4.75	100
0.15	89
0.425	63
0.063	57
0.04	41
0.02	35
0.01	20
0.001	6

Natural moisture content = 8%

Liquid Limit = 10; Plastic Limit = 7

- i) Determine Gravel, Sand, Silt and Clay fractions. (6 points)
- ii) Classify the soil based on USCS. (4 points)

6. Figure B6 shows a pad foundation transmitting its column load to ground.

- a) Sketch the pressure bulb that demarcates the zone of influence corresponding to 10% of footing pressure. State important dimensions. (6 points)
- b) Hence estimate the depth of investigation to ensure that the footing may not undergo excessive settlement. (2 points)

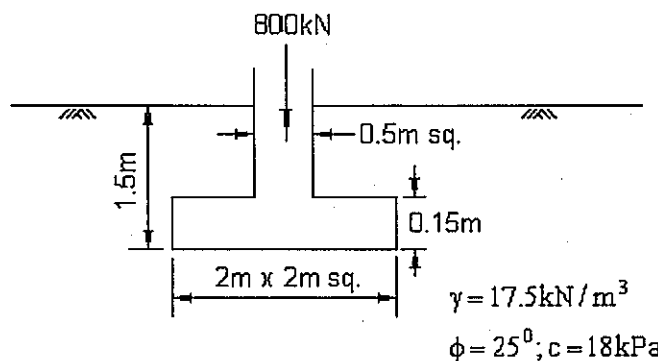


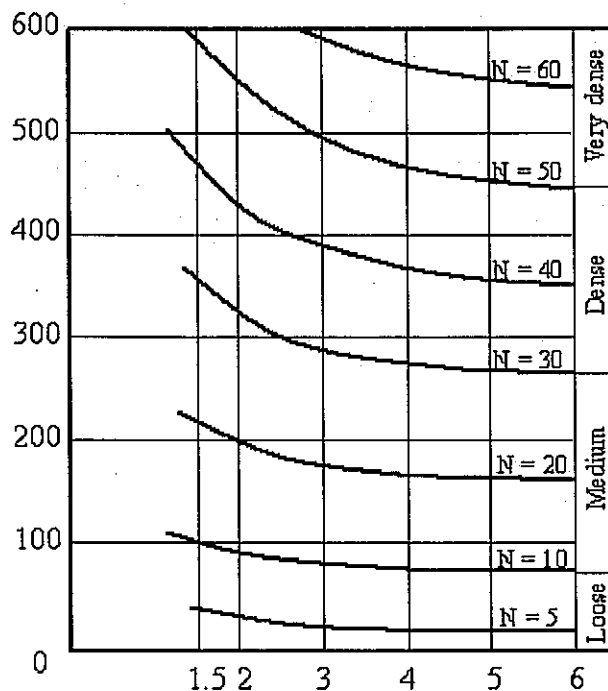
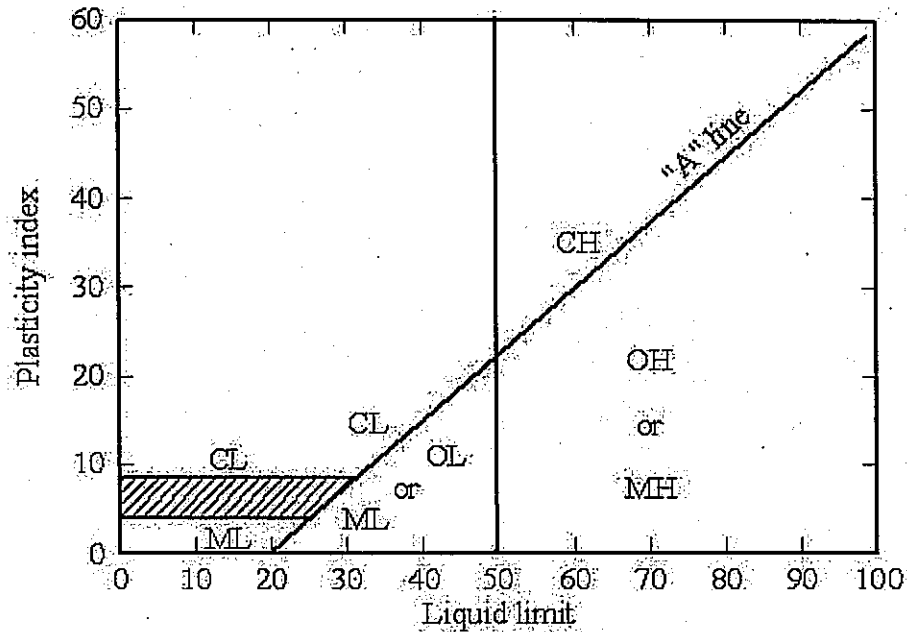
Figure B6

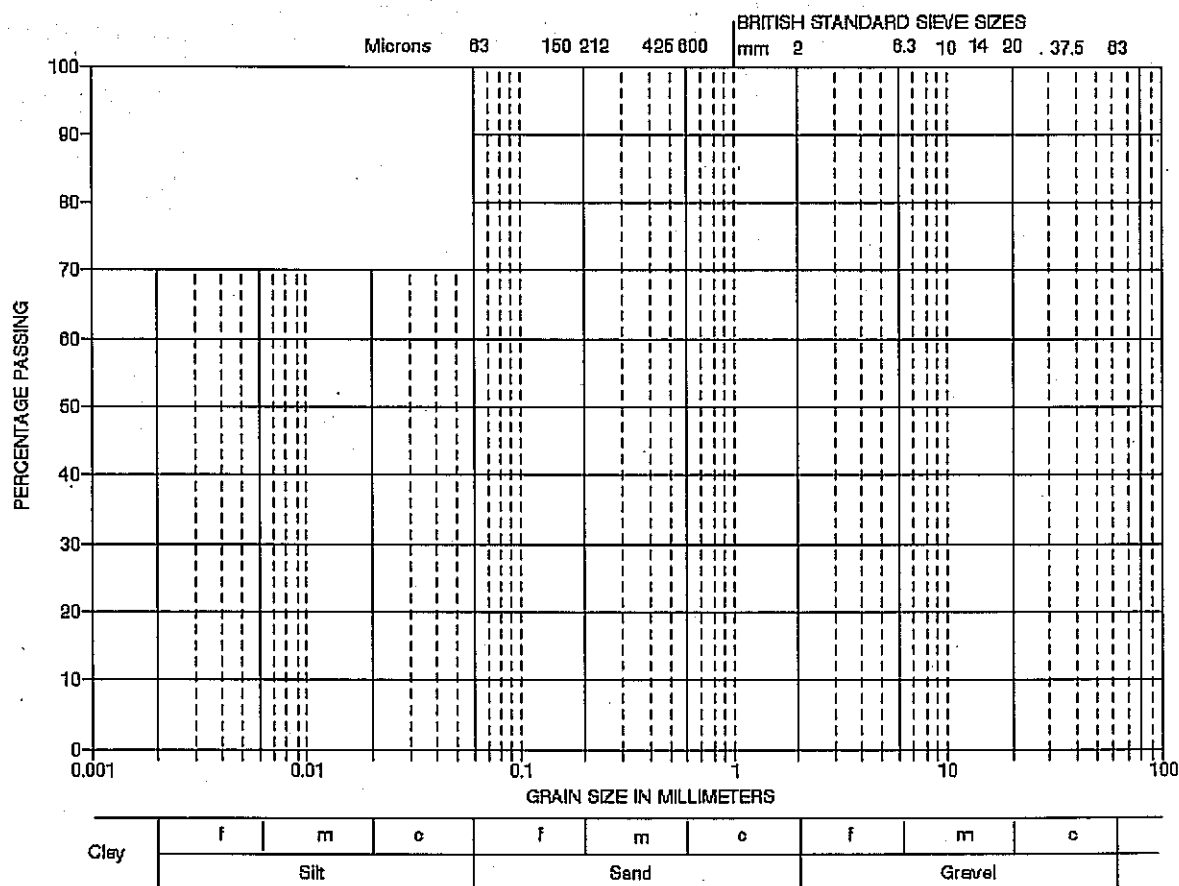
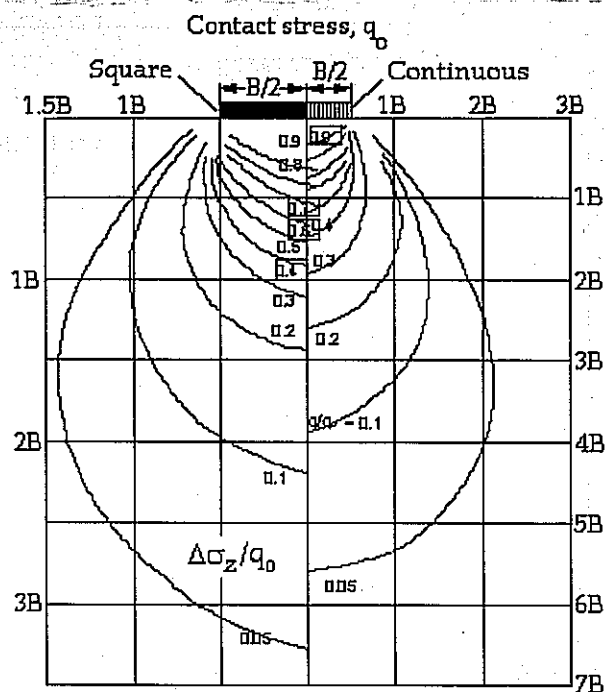
- c) Compute the Ultimate Bearing Capacity for the given footing if the super-structure load is 800kN. A suitable unit weight for concrete can be assumed. The Ultimate Bearing Capacity of a square footing is expressed as $q_u = 1.3cN_c + qN_q + 0.4\gamma BN_\gamma$. (6 points)

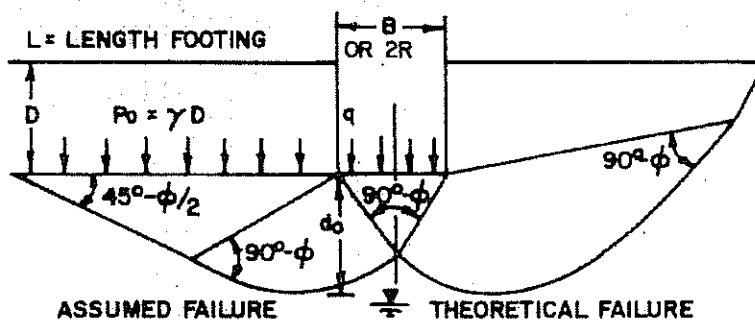
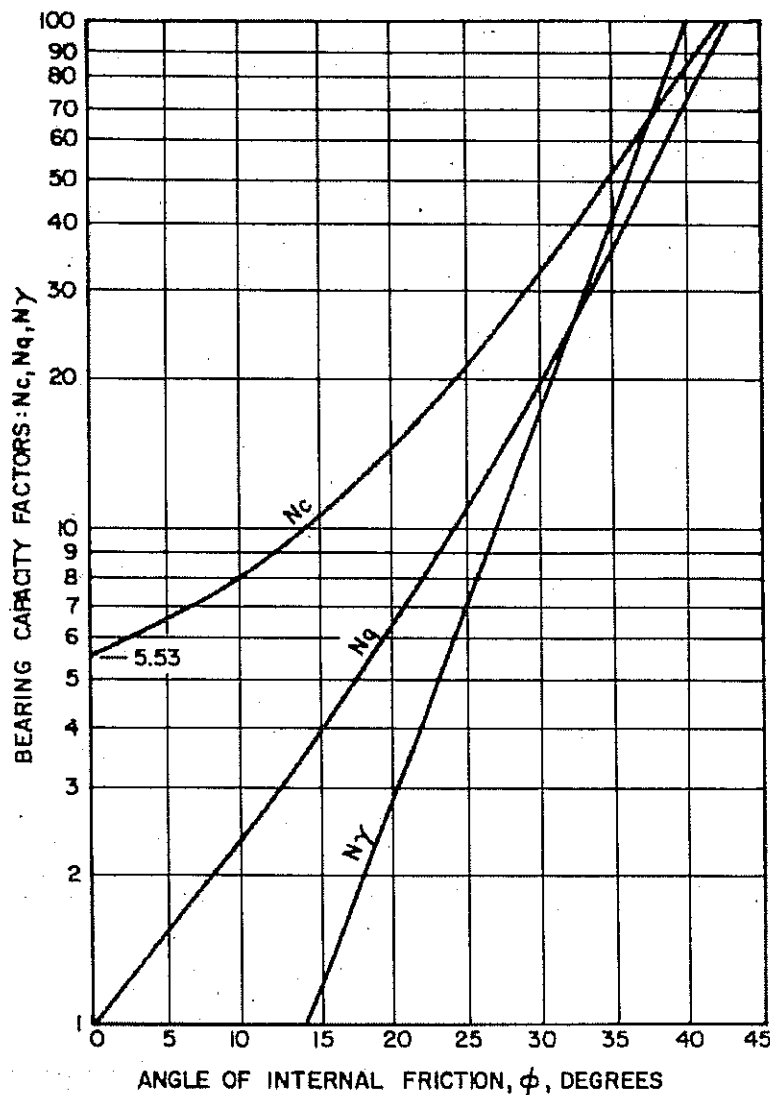
d) Compute the Factor of Safety against ultimate failure.

7. Explain the following mechanisms by providing a sketch with explanatory notes.

- The state of a sandy soil surrounding a split barrel sampler when it is driven in, during the Standard Penetration Test. (4 points)
- The state of a sandy soil specimen in a constant head permeameter, when it reaches its critical hydraulic gradient. (4 points)
- State of a soil element at an active state of plastic equilibrium. (4 points)
- A fine grained soil that shows a high dilatancy reaction. (4 points)







ASSUMED CONDITIONS:

1. $D \leq B$
2. SOIL IS UNIFORM TO DEPTH $d_0 > B$.
3. WATER LEVEL LOWER THAN d_0 BELOW BASE OF FOOTING.
4. VERTICAL LOAD CONCENTRIC.
5. FRICTION AND ADHESION ON VERTICAL SIDES OF FOOTING ARE NEGLECTED.
6. FOUNDATION SOIL WITH PROPERTIES c, ϕ, γ

ULTIMATE BEARING CAPACITY = q_{ult}

CONTINUOUS FOOTING; GENERAL CASE

$$q_{ult} = q' + q''$$

q' = PORTION OF BEARING

CAPACITY ASSUMING

WEIGHTLESS FOUNDATION SOIL

q'' = PORTION OF BEARING

CAPACITY FROM WEIGHT OF

FOUNDATION SOILS

$$q' = cN_c + \gamma DN_q$$

$$q'' = \gamma \frac{B}{2} N_y$$

$$q_{ult} = cN_c + \gamma DN_q + \frac{\gamma B}{2} N_y$$

SQUARE OR RECTANGULAR FOOTING

$$q_{ult} = cN_c \left(1 + 3 \frac{B}{L}\right) + \gamma DN_q + 0.4 \gamma B N_y$$

CIRCULAR FOOTING: $R = B/2$

$$q_{ult} = 1.3 cN_c + \gamma DN_q + 0.6 \gamma R N_y$$

FOR COHESIONLESS FOUNDATION
SOILS ($c = 0$)

CONTINUOUS FOOTING:

$$q_{ult} = \gamma DN_q + \frac{\gamma B}{2} N_y$$

SQUARE OR RECTANGULAR FOOTING:

$$q_{ult} = \gamma DN_q + 0.4 \gamma B N_y$$

CIRCULAR FOOTING:

$$q_{ult} = \gamma DN_q + 0.6 \gamma R N_y$$

FOR COHESIVE FOUNDATION
SOILS ($\phi = 0$)

CONTINUOUS FOOTING:

$$q_{ult} = cN_c + \gamma D$$

SQUARE OR RECTANGULAR FOOTING:

$$q_{ult} = cN_c \left(1 + 3 \frac{B}{L}\right) + \gamma D$$

CIRCULAR FOOTING:

$$q_{ult} = 1.3 cN_c + \gamma D$$